#### PROBLEM SET 3 - SOLUTIONS

#### **Comments on Problem Set 4**

#### **PROBLEM 1:**

- Be careful with the directions of the "other forces" when you apply momentum conservation in a control volume. Typically, you draw and calculate the forces exerted by the surroundings on the control volume of fluid. At the end, you need calculate the forces exerted by the fluid on the surroundings (in this case on the pipe), which have the same magnitude as the forces on the fluid but opposite direction. Many groups calculated " $F_x$ " and " $F_y$ " without specifying if those were the forces on the fluid or the forces on the pipe, and some groups got confused because of this.
- Onto the same issue, the best way to specify the direction of a force is to show it in a sketch.

#### **PROBLEM 2:**

- Remember that we have seen two ways of applying conservation of energy. First, you can apply Bernoulli between two points along a streamline. This is our good old expression:

$$z_1 + p_1/(\rho g) + v_1^2/(2g) = z_2 + p_2/(\rho g) + v_2^2/(2g) + (losses between 1 and 2)$$

where  $z_1$ ,  $p_1$ , and  $v_1$  are the elevation, pressure, and velocity at point 1 (same for point 2). We used this many times in the old times (a.k.a. before test 1) when we assumed inviscid flow and didn't have losses. Now, we are dealing with real flows, in which there are usually losses. Furthermore, these real flows are not usually uniform in the cross-section, and we are not usually interested in determining velocities at specific points of the cross-section. Rather, we are interested in determining the "cross-sectional averaged" properties of the flow, and thus we work with the cross-sectional average velocity, V. For this reason, we have developed the control volume analysis, which deals with cross-sectional averaged quantities. The conservation of energy between the inflow and the outflow of a Control Volume reads

$$z_{1,CG} + p_{1,CG}/(\rho g) + {V_1}^2/(2g) = z_{2,CG} + p_{2,CG}/(\rho g) + {V_2}^2/(2g) + (losses \ between \ 1 \ and \ 2)$$

It looks similar to our old good Bernoulli, but it is conceptually different! Now we apply conservation of energy between two sections (not two points), which are the inflow and the outflow of our Control Volume.  $z_{1,CG}$  is the elevation of the center of gravity of section 1,  $p_{1,CG}$  the pressure at the center of gravity of section 1, and  $V_1$  the average velocity in section 1 (not the velocity at a particular point). So, in the future, be explicit about which version of Bernoulli you are applying, and between which and which section (or which and which point).

NOTE: To apply conservation principles, the control volume must always be chosen so that the flow is well-behaved both in the inflow and the outflow. For this reason, pressure is hydrostatic at these sections. Therefore, the piezometric head,  $z_{1,CG} + p_{1,CG}/(\rho g)$ , is constant in all the points on the inflow section (and same for the outflow), and you can evaluate the sum of these two terms at any point, not necessarily at the center of gravity.

#### **PROBLEM 3:**

- Notice that the reason why we can neglect the headloss in problems 1 and 3 is because in both cases we have a short transition of a converging flow. Since the transition is short, the friction loss is very small. Since the flow is always converging, there is no minor loss.

#### Problem #4:

A few groups didn't have the formula for head loss quite right – we must also take into account the elevation difference from one side of the hydraulic jump to the other. The formula can easily be derived from the equation:  $H_1 = H_2 + HL_{1-->2}$ . See solutions for details. A handful of groups also answered the last part incorrectly. Conceptually, if we lose energy going from 1 --> 2, we'd have to gain energy if we went from 2 --> 1 (by conservation of energy). This would yield a negative head loss and negative energy dissipation (which implies a head gain and energy gain). This situation is physically impossible, so the flow only goes from 1 to 2. See solution for mathematical solutions.

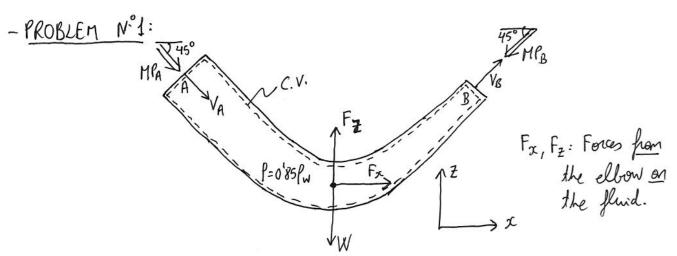
#### Problem #5:

Don't ever assume that two graphs are the same simply by looking at a single plot! They may look the same with the scale you've used, but there still may be large errors. Such is the case in this problem for values of y near the bottom (y = 0) where errors reach 25%. Everyone lost a point or two on this. Also, use Excel or another graphing program to make your plots. Handwritten plots are not acceptable (especially when units and other things are missing), especially for something like this where you have to be very accurate. If you don't know how to make graphs in Excel, ask the TAs. It's an important skill that you'll be able to use throughout your MIT life and beyond.

#### Problem #6:

Don't forget to explicitly say which way the force is acting.

# PROBLEM SET 4-SOLUTIONS



Conservation of volume between A and B:

$$Q_{A} = Q_{B} \implies V_{A} \cdot A_{A} = V_{B} \cdot A_{B} \implies V_{B} = \frac{A_{A}}{A_{B}} V_{A}$$

$$A_{A} = \frac{\pi d_{A}^{2}}{4} = \frac{\pi \cdot 0'4^{2}}{4} = 0'1257 m^{2}$$

$$A_{B} = \frac{\pi d_{B}^{2}}{4} = \frac{\pi \cdot 0'2^{2}}{4} = 0'03142 m^{2}$$

$$V_{B} = \frac{0'1257}{0'03142} V_{A} = 4 V_{A}$$

Bernoulli between A and B (We reglect losses, which moves une if the distance between A and B is small, because then we have a short transition of a converging flow).

$$Z_A = Z_B$$
,  $V_B = 4V_A$ 

$$\frac{1}{2} \int (4^{2} - 1) V_{A}^{2} = h_{A} - h_{B}$$

$$\frac{1}{2} 850 \cdot 15 \cdot V_{A}^{2} = 15 \cdot 10^{5} - 13 \cdot 10^{5} \implies V_{A} = 1771 \text{ m/s}$$

Thurt on A:  $MP_{A} = (PV_{A}^{2} + p_{CG,A}) \cdot A_{A} = (850 \cdot 1'771^{2} + 1'5 \cdot 10^{5}) \cdot 0'1257 = 19190 \text{ N}$ Thurt on B:  $MP_{B} = (PV_{B}^{2} + p_{CG_{1}B}) \cdot A_{B} = (850 \cdot 7'085^{2} + 1'3 \cdot 10^{5}) \cdot 0'03142 = 5425 \text{ N}$  W = 17

Weight:

W = fg V = 850. 9'8. 0'15 = 1250 N

Conservation of momentum in x-direction:

2 Forces in x = MPA cos 45° - MPB cos 45° + Fx=0=)

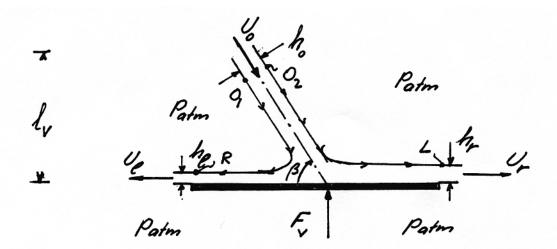
=)  $f_{x} = -19190 \frac{\sqrt{2}}{2} + 5425 \frac{\sqrt{2}}{2} = -\frac{9733}{2} \text{ N}$  (to the left)

The horizantal force from the fluid on the elbow is of the same magnitude acting to the right.

Conservation of momentum in z-direction:

The vertical force from the fluid on the elbow is of the rane magnitude acting downwards.

### - PROBLEM No. 2:



a) Ue and Up are horizontal, and therefore conhibute no momentum force in vertical direction. The only vertical momentum force is possible incident jet

(9 6 ho + Poho) sing = F

but po = 0 [atmospheric pressure on all sides],

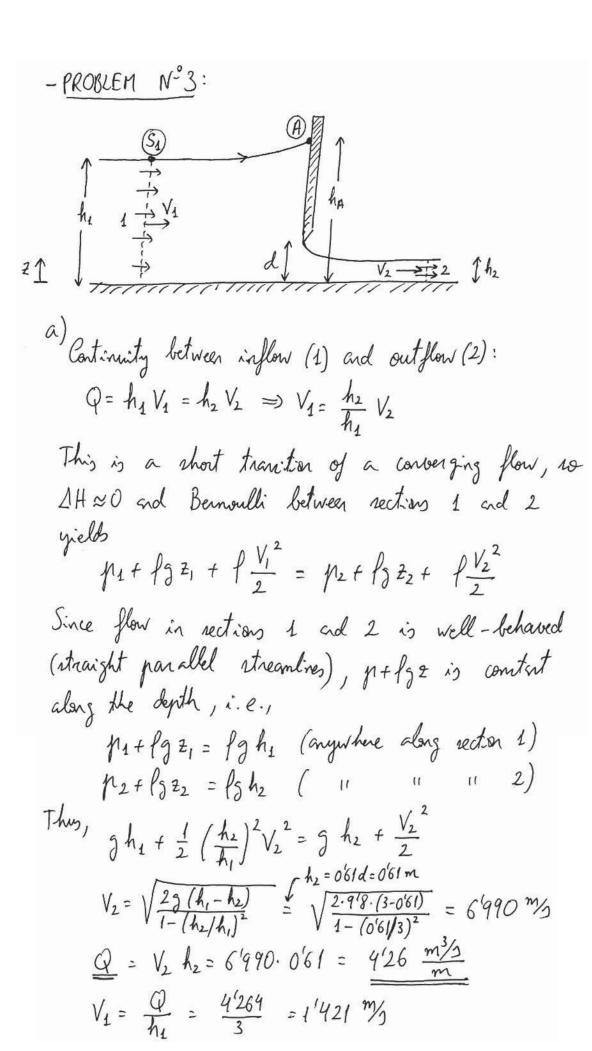
Vertical jet force is downward = ploho sing

b) Horizontal momentum balance gives, with all pressures being zero since jets are pee

gue he + guh cosp = guh

or since Ue = Uo = Ur

hr - he = ho cosp (1) Conservation of volume (continuity) gives Uho = Qin = Urhr + Ue he = I Quit hr + he = ho (2) Combining (1) and (2) gives h\_=h(1+cops)/2; hp=h(1-cops)/2 c) Application of Bennoulli along streamline from 0, to R gives U. 129 + P. 199 + 20 = U, 129 + A/99 +2but Po = Pr = 0 and Zo - Zr is negligible, so U2 = U2 Similarly, Bennoulli from 02 to L gives Uo = Ue (= Ur pom above) g.e.d. From (c) we have along sheamline from 0, to R U-129 +(2, -2,) = U-129 Magnitude of U/2g = 14 / (2.10) = 10m If  $(2_0-2_r)=l_v=1$  m  $U_r=14^2+29=U_r=14.7 \frac{m}{s}$  with gravity, 14 m/s without. Difference  $\approx 5\%$ I would start to worry if ly > 1 to 2m



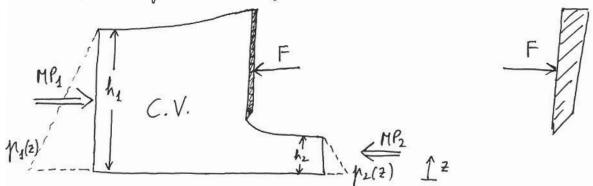
Now we apply Bernoulli along the streamline from 
$$S_1$$
 to  $\widehat{A}$ :

 $\eta_{S_1} + f_{\widehat{G}} h_1 + \frac{1}{2} f V_1^2 = \eta_A + f_{\widehat{G}} h_A + \frac{1}{2} f v_A^2$ 
 $v_A = 0 \text{ rise } \widehat{A}$ 

is a convex corner

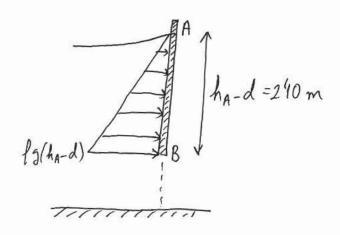
$$\frac{h_A}{=} h_1 + \frac{V_1^2}{25} = 3 + \frac{1'421^2}{2.9'8} = \frac{3'10 \text{ m}}{}$$

To calculate the horizontal force on the gate, we'll apply conservation of linear momentum in x-direction to the control volume between sections 1 and 2: (Bottom friction is neglected since it is a "short" transition)



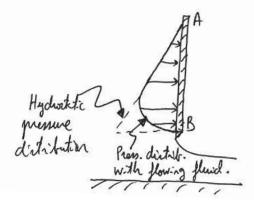
$$\begin{split} \text{MP}_{1} &= \left( P_{CG_{1}} + P_{V_{1}}^{2} \right) h_{1} = \left( \frac{P_{2} h_{1}}{2} + P_{V_{1}}^{2} \right) h_{1} = \left( \frac{9800 \cdot 3}{2} + 1000 \cdot 1'421^{2} \right) \cdot 3^{2} \\ \text{L well behard} \\ \text{MP}_{2} &= \left( \frac{P_{2} h_{2}}{2} + P_{V_{2}}^{2} \right) h_{2} = \left( \frac{9800 \cdot 0'61}{2} + 1000 \cdot 6'990^{2} \right) \cdot 0'61 = 31628 \, \text{N/m} \\ \text{EF}_{x} &= 0 \quad \left( \text{steedy flow} \right) \Rightarrow \quad \text{MP}_{1} - \text{F-MP}_{2} = 0 \Rightarrow \quad \text{F= MP}_{1} - \text{MP}_{2} = 18530 \, \text{N/m} \\ \left( \text{force from fluid on gate} \right) \\ \text{acts to the right} \end{split}$$

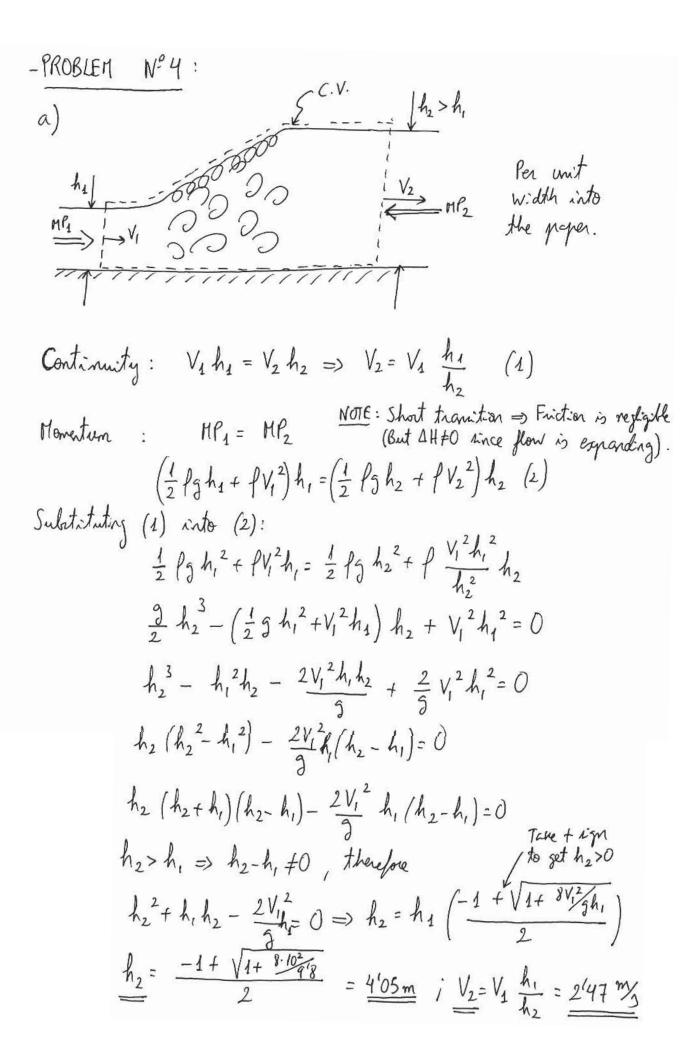
d) assuming hydrostatic pressure:



 $F = \text{ area of the triangle of pressure} = \frac{1}{2} lg (h_A - d)^2 = \frac{9800}{2} \cdot 2'10^2 = \frac{21609}{100} \frac{1}{2}$ 

When flow is moving part the gate, there is velocity near the bottom of the gate. Therefore, the pressure will be reduced near the bottom of the gate with respect to hydrostatic pressure (this is suggested by Bernoulli equation). Note also that point B is in contact with the atmosphere, so  $p_B=0$ . Thus, the total force on the gate (18530 ½m) is smaller than in the hydrostatic case (21609 ½m).





b) Evergy conservation:  $\frac{V_1^2}{2g} + \frac{\mu_1}{\ell g} + Z_1 = \frac{V_2^2}{2g} + \frac{\mu_2}{\ell g} + Z_2 + 2H_{1 \to 2}$ Well-behaved flow in sections 1 and 2 => to + 2, = content = hs 12 + Z2 = constant = h2 Thus,  $\underline{\Delta H_{4\to 2}} = \frac{V_1^2 - V_2^2}{25} + (h_1 - h_2) = \frac{10^2 - 2'47^2}{2 \cdot 9'8} + (1 - 4'05) = \underline{1'75} \text{ m}$ Rate of energy discipation per unit width of the channel: E1-2 = Pg QAH 1-2 = 9800 · (10.1) · 1'75 = 1'72.105 Tms () To obtain he and Ve from he and Ve, we need to apply continuity and momentum, as we did in (a). The two exactions remain unchanged when we revene flow direction. Therefore, if he and Ve have the same values as in (a), h, and Ve will also have the same values, that is: hi= 1 m and Vi=10 %. d) New energy conservation yields: compare with (b)  $\frac{V_2^-}{2g} + \frac{\eta_2}{\ell g} + \frac{z}{2} = \frac{V_1^-}{2g} + \frac{\eta_1}{\ell g} + \frac{z}{\ell f} + \Delta H_{2\rightarrow 1} \Rightarrow \underline{\Delta H_{2\rightarrow 1}} = -\Delta H_{1\rightarrow 2} = -\frac{1}{75} m < 0!$ and Ez+1 = 13 QAHz+1 = - 1/72.105 /ms <0! Obviously, energy dissipation cannot be negative, since this would mean creation of energy. Therefore, we conclude that this hypothetical flow from right to left is impossible. This conclusion is true for any values of he, Ve such that he < he: In a hydraulic jump, flow always goes from smaller to larger depth. This is consistent with the fact that we expect energy dissipation in flow expansions and not in (smooth) flow contractions. Since the hydraulic jump introduces turbulent dissipction, it should correspond to

a flow expansion.

a)
$$U = \frac{1}{h} \int_{0}^{h} u(y) dy = \frac{u_{3}}{h^{1+1/n}} \int_{0}^{h} y^{1/n} dy =$$

$$= \frac{u_{3}}{h^{\frac{n+1}{n}}} \left[ \frac{n}{n+1} y^{\frac{n+1}{n}} \right]_{0}^{h} = \frac{u_{3}}{h^{\frac{n+1}{n}}} h^{\frac{n+1}{n}} = \frac{n}{n+1} u_{3}$$

b) The momentum coefficient is defined as

$$k_m = \frac{\int_A q_1^2 dA}{U^2 A}$$
;  $q_1 = u = U_2 \left(\frac{y}{h}\right)^m$ ;  $U = average$ 

Dividing the numerator and the denominator by the width of the channel,

$$\frac{K_m}{u_0^2 \left(\frac{y}{n}\right)^2 h} = \left(\frac{n+1}{n}\right)^2 \frac{1}{h^{1+2h}} \int_0^h y^{2h} dh = \frac{1}{n} \left(\frac{n+1}{n}\right)^2 h = \frac{1}{n} \left(\frac{n+1}{n}\right)^2 h$$

$$= \left(\frac{n+1}{n}\right)^2 \frac{1}{h^{1+2/n}} \frac{1}{1+2/n} h^{1+2/n} = \frac{(n+1)^2}{n(n+2)}$$

c) The energy coefficient is defined as  $\frac{k_e = \int_A q L^3 dA}{U^3 A} = \frac{\int_0^h u_3^3 \left(\frac{y}{h}\right)^{3/h} dy}{u_3^3 \left(\frac{n}{h}\right)^3 h} =$ 

$$= \left(\frac{n+1}{n}\right)^3 \frac{1}{h^{1+3/n}} \frac{1}{1+3/n} h^{1+3/n} = \frac{(n+1)^3}{n^2(n+3)}$$

d) From PROBLEM SET 2, PROBLEM Nº 4:

$$U = 0'1 \ln \left( \frac{y + 3 \cdot 10^{-4}}{3 \cdot 10^{-4}} \right) \quad 0 \le y \le 2 \quad (5.i.)$$

$$U_3 = U \left( y = 2 \right) = 0'1 \ln \left( \frac{2 + 3 \cdot 10^{-4}}{3 \cdot 10^{-4}} \right) = 0'881 \text{ m/s}$$

$$U = 0'780 \text{ m/s} \quad (\text{from } PS2)$$

$$h = 2 \text{ m}$$

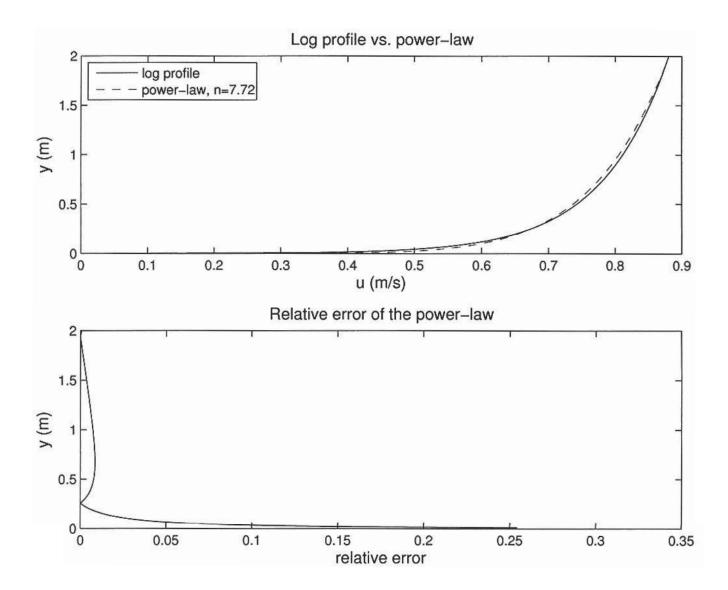
Denote the power-law approximation with a tilde (~):

ũs= Us= 0'881 m/s

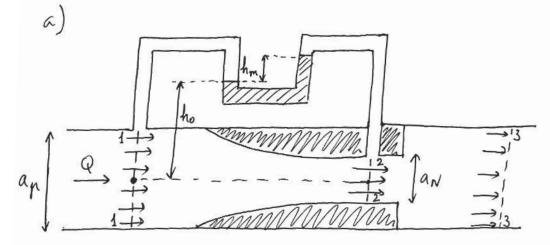
$$\tilde{U} = U \Rightarrow \frac{n}{n+1} \tilde{u}_{5} = \frac{n}{n+1} \cdot 0'88l = 0'780 \Rightarrow n = 7'72$$

On rest page, I have represented the log profile is. the power-law approximation, as well as the relative even of the power-law (defined as  $\left|\frac{\widetilde{u}(y) - u(y)}{u(y)}\right|$ ). As seen in the figure, the agreement is good. The power-law profile slightly underpredicts the log profile (even  $\approx 1\%$ ) in the upper region, while it yields a 25% observediction of the near-bottom velocity. Other than in the near-bottom (relevant, e.g., for rediment transport calculations), the agreement is very good.

e)  $\underline{\underline{K_m}} = \frac{(7'72+1)^2}{7'72(7'72+2)} = \underline{\underline{1'013}} \quad j \quad \underline{\underline{Ke}} = \frac{(7'72+1)^3}{7'72^2(7'72+3)} = \underline{\underline{1'038}}$ Both have value close to 1, so the momentum and everyy of the flow are well represented by the average velocity values. These results are also consistent with what we saw in excitation 5:  $\delta^2 \approx 0'013 \rightarrow \text{ Km} = 1 + \delta^2, \text{ Ke} = 1 + 3\delta^2.$ 



## - PROBLEM Nº6:



At 1-1 and 2-2 we have well-behaved flow and prossure is hydrostatic. With this, from the manometer reading, we can obtain the pressure difference between 1 and 2:

1, (G-fgho-fmghm+fg(hm+ho)=p2, (E)

=> p1,c0 - p2,c6 = (fm-f) g hm = 12600. 9'8.0'061 = 7532 Pa

The CG of sections 1 and 2 are on the same streamline. Neglecting wall friction effects (since transition is short), we apply Bernoulli between 1-1 and 2-2 considering the center streamline:

 $p_{1}$ ,  $c_{6} + \frac{1}{2} \int V_{1}^{2} = p_{2}$ ,  $c_{6} + \frac{1}{2} \int V_{2}^{2}$  (1) Where  $V_{1} = Q/A_{1}$  and  $V_{2} = Q/A_{2}$  ( $K_{m} = Ke = 1$  assumed) and we have applied that  $Z_{1}$ ,  $c_{6} = Z_{2}$ ,  $c_{6}$ . Continuity dictates:

 $Q = V_1 a_{N}^2 = V_2 a_{N}^2 \Rightarrow V_2 = \frac{a_{N}^2}{a_{N}^2} V_1 = 4V_1$  (2)

Plugging (2) into (1):

$$M_{1,CG} + \frac{1}{2} \int V_{1}^{2} = M_{2,G} + 8 \int V_{1}^{2} = V_{2} = \sqrt{\frac{2(M_{1,CG} - M_{2,CG})}{15 f}} = \sqrt{\frac{2.7532}{15 \cdot 1000}} = 100 \text{ m/s}$$
 $V_{2} = 4'00 \text{ m/s}, \quad Q = V_{3} \text{ ap}^{2} = 1.01^{2} = 001 \text{ m/s}$ 

b) Go seen in decture 13, the headlows in an abupt expansion is

$$AH \exp = \frac{(V_{2} - V_{3})^{2}}{2g}$$
By continuity,  $Q = V_{1} \text{ ap}^{2} = V_{3} \text{ ap}^{2} = V_{3} = V_{1}$ . Therefore

$$AH \exp = \frac{(4-1)^{2}}{2 \cdot 9'8} = 0'459 \text{ m}$$

c)

$$AH_{2} = \frac{(4-1)^{2}}{2 \cdot 9'8} = 0'459 \text{ m}$$

c)

$$AH_{3} = \frac{(4-1)^{2}}{2 \cdot 9'8} = 0'459 \text{ m}$$

c)

$$AH_{4} = \frac{(4-1)^{2}}{2 \cdot 9'8} = 0'459 \text{ m}$$

c)

$$AH_{5} = \frac{(4-1)^{2}}{2 \cdot 9'8} = 0'459 \text{ m}$$

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The following is shorth.

All expression of every between 1-1 and 3-3 dictales.

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